

X

इंटरनेट

Disclosure to Promote the Right To Information

Whereas the Parliament of India has set out to provide a practical regime of right to information for citizens to secure access to information under the control of public authorities, in order to promote transparency and accountability in the working of every public authority, and whereas the attached publication of the Bureau of Indian Standards is of particular interest to the public, particularly disadvantaged communities and those engaged in the pursuit of education and knowledge, the attached public safety standard is made available to promote the timely dissemination of this information in an accurate manner to the public.

"जानने का अधिकार, जीने का अधिकार" Mazdoor Kisan Shakti Sangathan "The Right to Information, The Right to Live"

"पुराने को छोड नये के तरफ" Jawaharlal Nehru "Step Out From the Old to the New"

मानक

IS 9221 (1979): Method for the determination of modulus of

elasticity and Poisson's ratio of rock materials in uniaxial compression [CED 48: Rock Mechanics]



Made Available By

Public.Resource.Org RIGHT TO INFORMATION "ज्ञान एक ऐसा खजाना है जो कभी चुराया नहीं जा सकता Bhartrhari-Nītiśatakam

"Knowledge is such a treasure which cannot be stolen"

"ज्ञान से एक नये भारत का निर्माण″ Satyanarayan Gangaram Pitroda "Invent a New India Using Knowledge"



BLANK PAGE



PROTECTED BY COPYRIGHT

"पुनर्फ़ः १६६६" "RE_AFFIRMED 1996" 15:9221 - 1979

Indian Standard METHOD FOR DETERMINATION OF MODULUS OF ELASTICITY AND POISSON'S RATIO OF ROCK MATERIALS IN UNIAXIAL COMPRESSION

UDC 624.121.372 : 539.32



Copyright 1979

INDIAN STANDARDS INSTITUTION MANAK BHAVAN. 9 BAHADUR SHAH ZAFAR MARG NEW DELHI 110002



November 1979

Indian Standard METHOD FOR DETERMINATION OF MODULUS OF ELASTICITY AND POISSON'S RATIO OF ROCK MATERIALS IN UNIAXIAL COMPRESSION

Soil Engineering and Rock Mechanics Sectional Committee, BDC 23

Chairman	Representing
Prof Dinesh Mohan	Central Building Research Institute (CSIR), Roorkee
Members	
Additional Director Research (FE), RDSO DEPUTY DIRECTOR RESEARCH	Railway Board (Ministry of Railways)
(FE-I), RDSO (Alternate) PROF ALAM SINGH LT-COL AVTAR SINGH	University of Jodhpur, Jodhpur Engineer-in-Chief's Branch, Army Headquarters
MAJ V. K. KANITKAR (Alternate Dr A. BANERJEE SHRI S. GUPTA (Alternate)	Cementation Co Ltd, Calcutta
Dr R. K. Bhandari Chief Engineer (D & R)	Central Building Research Institute (CSIR), Roorkee Irrigation Department, Government of Punjab, Chandigarh
DIRECTOR (IPRI) (Alternate) SHRI K. N. DADINA	In personal capacity (P-820 New Alipore, Calcutta 700053)
Shri A. G. Dastidar	In personal capacity (5, Hungerford Street, 12/1 Hun- gerford Court, Calcutta 700017)
DR G. S. DHILLON DIRECTOR (CSMRS) DEPUTY DIRECTOR (CSMRS) (Alternate)	Indian Geotechnical Society, New Delhi Central Water Commission, New Delhi
SHRI A. H. DIVANJI SHRI A. N. JANGLE (Alternate)	Asia Foundations & Construction (P) Ltd, Bombay
Dr Gopal Ranjan	University of Roorkee, Roorkee; and Institution of Engineers (India) (Delhi Centre)
Dr Shashi K. Gulhati Dr G. V. Rao (Alternate)	Indian Institute of Technology, New Delhi
SHRI O. P. MALHOTRA	Public Works Department, Government of Punjab, Chandigarh
Shri T. K. Natarajan	Central Road Research Institute (CSIR), New Delhi
	(Continued on page 2)

Copyright 1979

INDIAN STANDARDS INSTITUTION

This publication is protected under the Indian Copyright Act (XIV of 1957) and reproduction in whole or in part by any means except with written permission of the publisher shall be deemed to be an infringement of copyright under the said Act.

(Continued from page 1)

Members

REPRESENTATIVE RESEARCH OFFICER SHRI K. R. SAXENA SECRETARY DEPUTY SECRETARY (Alternate) SHRI M. M. D. SETH

DR B. L. DHAWAN (Alternate) SHRI M. K. SINGHAL SHRI N. SIVAGURU SHRI D.V. SIKKA (Alternate) SHRI K. S. SRINIVASAN SHRI SUNIL BERRY (Alternate) SUPERINTENDING ENGINEER (P & D Representing

Irrigation Research Institute, Khagaul, Patna Building & Roads Research Laboratory, Chandigarh Engineering Research Laboratories, Hyderabad Central Board of Irrigation & Power, New Delhi

Public Works Department, Government of Uttar Pradesh

Irrigation Research Institute, Roorkee Roads Wings (Ministry of Shipping & Transport)

National Buildings Organization, New Delhi

SUPERINTENDING ENGINEER (P & D) Public Works Department, Government of Tamil Nadu, Madras

Executive Engineer (SMRD) (Alternate) Shri B. T. UNWALLA Shri T. M. MENON (Alternate) Shri H. C. VERMA

SHRI V. S. VASUDEVAN (Alternate) SHRI D. AJITHA SIMHA, Director (Civ Engg)

Concrete Association of India, Bombay

Director General, ISI (Ex-officio Member)

All India Instruments Manufacturers & Dealers Association, Bombay

Secretary

Shri K. M. Mathur

Deputy Director (Civ Engg), ISI

Rock Mechanics Subcommittee, BDC 23:5

Convener

Shri R. S. Melkote	Central Water Commission, New Delhi
Members	
Director	Central Water & Power Research Station, Pune
SHRI S. L. MOKHASHI (Alternate)	
DIRECTOR	Maharashtra Engineering Research Institute, Nasik
RESEARCH OFFICER (Alternate)	
SHRI B. K. KAUL	Kurukshetra University, Kurukshetra
Shri P. L. Narula	Geological Survey of India, Lucknow
Dr T. Ramamurthi	Indian Institute of Technology, New Delhi
Dr Y. V. Ramana	National Geophysical Research Institute, Hyderabad
Secretary	Central Board of Irrigation and Power, New Delhi
DEPUTY SECRETARY (Alternate)	
DR B. SINGH	Central Mining Research Station, Dhanbad
SHRI A. K. DUBE (Alternate)	
Shri H. C. Verma	Associated Instruments Manufacturers (I) Pvt Ltd, New Delhi
PROF T. S. NAGARAJ (Alternate)	

2

Indian Standard

METHOD FOR DETERMINATION OF MODULUS OF ELASTICITY AND POISSON'S RATIO OF ROCK MATERIALS IN UNIAXIAL COMPRESSION

0. FOREWORD

0.1 This Indian Standard was adopted by the Indian Standards Institution on 12 June 1979, after the draft finalized by the Soil Engineering and Rock Mechanics Sectional Committee had been approved by the Civil Engineering Division Council.

0.2 A number of Indian Standards covering the method for tests of rock materials are being formulated. In this standard method for determination of modulus of elasticity and Poisson's ratio of rock materials in uniaxial compression is covered.

0.3 In the formulation of this standard due weightage has been given to international co-ordination among the standards and practices prevailing in different countries in addition to relating it to the practices in the field in this country.

0.4 In reporting the result of a test or analysis made in accordance with this standard, if the final value, observed or calculated, is to be rounded off, it shall be done in accordance with IS: 2-1960*.

1. SCOPE

1.1 This standard covers the method for determination of Young's modulus of elasticity and Poisson's ratio of cylindrical rock specimens in compression.

1.2 This standard does not cover (a) the procedure necessary to obtain stress strain curve beyond the ultimate strength, and (b) the method for determining rock moduli in tension.

2. APPARATUS

2.1 A suitable loading machine shall be used for applying and measuring the axial load to the specimen. The capacity of the testing machine should

^{*}Rules for rounding off numerical values (revised).

IS: 9221 - 1979

be sufficient to assure reasonable longitudinal stiffness for size of the specimen tested. It shall provide adequate control on loading rate to suit the requirements as set in 4.3. It shall be verified and calibrated at suitable time intervals depending on the work load.

2.2 Discs made of steel having a hardness of not less than HRC 30 (see IS: 1586-1968*) shall be placed at specimen ends. The diameter of the discs should be same as the diameter of the specimen. The thickness of the discs shall be atleast 15 mm. Surfaces of the discs shall be ground and their flatness shall be within 025 mm.

Note --- With abrasive rocks, these discs tend to roughen after a number of specimens have been tested and hence need to be resurfaced from time to time.

2.3 One of the two discs shall incorporate a spherical seat. The spherical seat shall be placed on the upper end of the specimen. It shall be lightly lubricated with mineral oil. The specimen, the discs and the spherical seat shall be accurately centred with respect to one another and loading frame. The curvature centre of the seat surface should coincide with the centre of the top surface of the specimen.

2.4 Circumferential and axial deformations or strains may be determined from data obtained by electrical resistance strain gauges, compressometers, optical devices or other suitable means. The design of the measuring device shall be such that the average of atleast two circumferential and two axial strain measurements can be determined for each increment of load. Measuring positions shall be equally spaced around the circumference of the specimens close to the mid height. They should not fall within D/2 of the specimen ends, where D is the diameter. The length over which the strains are measured shall be atleast five times the grain size diameter, in magnitude. Both axial and circumferential strains shall be determined with an accuracy of 2 percent of the reading and a precision of 0.2 percent of the full scale.

3. TEST SPECIMENS

3.1 Test specimen shall be a right circular cylinder with tolerances as specified in **3.2** although specimen of any shape with regular geometry could be used and should be prepared as per IS: 9179-1979[†].

3.1.1 The specimen shall be tested at moisture contents as close to field conditions as possible.

3.2 Dimension of Specimen

3.2.1 The length to diameter ratio of the specimen should preferably be 2 to 3.

^{*}Method for Rockwell hardness test (B and C scales) for steel.

Method for preparation of rock specimen for laboratory testing.

3.2.2 The diameter of the specimen shall not be less than ten times the largest mineral grain in rock and preferably of 45 mm and in no case less than 30 mm. In the latter case the tolerances given in **3.2.3** and **3.2.5** be comparatively lowered.

3.2.3 Specimen ends shall be flat within 0.05 mm.

3.2.4 The ends shall be parallel to each other within 0.002D where D is the specimen diameter.

3.2.5 The ends shall be perpendicular to the axis of the specimen within 0.001 radians (3.5 minutes) or 0.05 mm in a 45 mm diameter specimen.

3.2.6 The cylindrical surface shall be smooth and free from abrupt irregularities and straight to within 0.3 mm over the full length of the specimen and the dimensions of the specimen shall not vary by more than 0.2 mm over the length of the specimen.

3.2.7 The diameter of the test specimen shall be measured to the nearest 0.1 mm by averaging two diameters measured at right angles to each other at about the upper height, the mid height and the lower height of the specimen. It shall not vary by more than 0.3 mm over the length of the specimen.

4. TEST PROCEDURE

4.1 The ability of spherical seat to rotate freely shall be checked before each test.

4.2 The surfaces of the two bearing discs and the test specimen shall be wiped clean. The specimen shall be kept on the lower disc. The axis of the specimen shall be carefully aligned with the centre of the thrust of the spherical seat. As the load is generally brought to bear on the specimen, the movable portion of the spherical seated disc shall be adjusted to ensure uniform seating.

4.3 Load on the specimen shall be applied continuously and without shock to produce an approximately constant rate of load or deformation such that failure will occur within 5 to 15 minutes of initiation of loading if carried to failure. Alternatively, the stress rate shall be within the limits of 0.5 Mega Pascal/s to 1 Mega Pascal/s. Load and the axial and diametral or circumferential strains shall be measured frequently at evenly spaced load intervals during the test. The maximum load on the specimen shall be recorded in kg within 1 percent. Atleast 10 readings should be taken over the load range to define the axial and diametric stress strain curves.

5

5. CALCULATION

5.1 The axial strain (ε_{e}) and the diametric strain (ε_{d}) may be recorded directly from strain indicating equipment or may be calculated from the measured deformation depending upon the type of apparatus or instrument used.

5.2 The axial (ε_a) and diametric (ε_d) strains shall be calculated as follows:

$$\begin{aligned} \mathbf{\varepsilon}_{\mathbf{s}} &= \Delta \mathbf{1}/\mathbf{1} \\ \mathbf{\varepsilon}_{\mathbf{d}} &= \Delta \mathbf{d}/\mathbf{d} \end{aligned}$$

where

l =original axial length before deformation,

d =original diameter before the deformation,

 Δ_1 = change in measured axial length (positive for a decrease in length), and

 Δd = change in diameter (positive for an increase in diameter).

NOTE — It may be noted that circumferentially applied electrical resistance strain gauges also reflect diametric strain, the value necessary for computing Poisson's ratio. Since,

$$C = \pi_d$$

and $\Delta_c = \pi \Delta_d$

The circumferential and diametral strains are related as follows:

$$\varepsilon_{c} = \Delta c/c$$

= $\pi \Delta a/\pi a$
= $\Delta d/a$
= ε_{d}

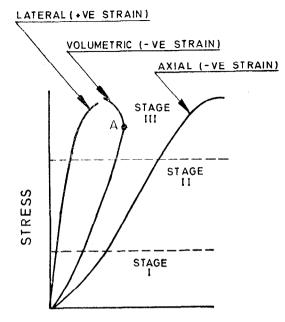
where C and d are circumference and diameter of the specimen respectively.

5.3 The compressive stress in the test specimen σ shall be calculated from compressive load P and the θ initially computed cross-sectional area A, as follows:

$$\sigma = \frac{P}{A}$$

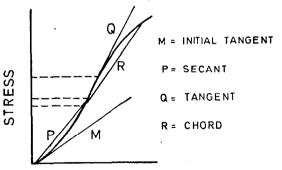
5.4 The stress versus axial and lateral strain shall be plotted as a curve (see Fig. 1).

5.5 The Young's modulus of elasticity E may be calculated using one of the several methods employed in engineering practice depending on the type of problem. The commonly used moduli are given in 5.5.1 to 5.5.3 (see Fig. 2).



STRAIN

- Stage I -- Internal cracking commences, closure of cracks and pores takes place
- Stage II Linear compression
- Stage III Internal cracking commences
 - FIG. 1 TYPICAL LATERAL, VOLUMETRIC AND AXIAL STRAIN DIAGRAMS FOR ROCK, IN COMPRESSION



STRAIN

Fig. 2 Definition of Moduli for a Non-Liner Stress-Strain Diagram

5.5.1 Tangent Modulus — Slope of the tangent at a given stress level, which is some fixed percentage of the maximum strength.

5.5.2 Chord Modulus — The average slope between two specified points defined by the chord joining them.

5.5.3 Secant Modulus — Chord modulus between origin and some other points on the curve.

5.6 Poisson's Ratio (v) — Poissons's ratio shall be calculated as the ratio of the total diametric strain ε_d to the total axial strain ε_a at any given stress level.

NOTE — When the terms 'Modulus' and 'Poisson's Ratio' are used without any qualification, they shall be taken to mean as the tangent modulus and the Poisson's ratio at 50 percent of the ultimate stress.

6. REPORT

6.1 The report shall give a plot of stress strain curve appropriate for the intended use, Young's modulus E and Poisson's ratio v with their method of determination and the stress levels.

6.2 The report of test shall include the following information:

- a) Number of specimens tested.
- b) Mode of failure.
- c) Lithological description of rock.
- d) Orientation of loading axis with respect to anisotropy, for example, bedding planes, foliations, etc.
- e) Source of sample, location, depth and orientation, date of sampling.
- f) Storage history and environment.
- g) Date of testing and type of machine used.
- h) Specimen diameter and height.
- j) Moisture content and room temperature.
- k) Duration of the test and stress rate.
- m) Other physical properties, such as specific gravity, absorption, permeability and porosity, citing their method of determination, if available.
- n) Any other observation.
- p) Should it be necessary to test specimens of size less than 45 mm or length to diameter ratio less than 2 to 3, suitable mention of these facts shall be made in the report.

INDIAN STANDARDS

ON

ROCK MECHANICS

IS:

7292-1974	Code of practice for in-situ determination of rock properties by flat jack
7317-1974	Code of practice for uniaxial jacking test for deformation modulus of rock
7746-1975	Code of practice for in-situ shear test on rock
8764-1978	Method for determination of point load strength index or rocks
9143-1979	Method for determination of unconfined compressive strength of rock materia
9179-1979	Method for preparation of rock specimen for laboratory testing

INDIAN STANDARDS INSTITUTION

1000-00

Manak Bhavan, 9 Bahadur Shah Zafar Marg, NEW DELHI 110002 Telephones : 26 60 21, 27 01 31 Telegrams : 1

Telegrams : Manaksanstha

ls

Regional Offices :	and the same and the strength	elephone
Western : Novelty Chambers, Grant Road Eastern : 5 Chowringhee Approach Southern : C. I. T. Campus, Adyar	BOMBAY 400007 CALCUTTA 700072 MADRAS 600020	37 97 29 23-08 02 41 24 42
Branch Offices :		
'Pushpak', Nurmohamed Shalkh Marg, Khanpur 'F' Block, Unity Bldg, Narasimharaja Square Gangotri Complex, Bhadbhada Road, T.T. Nagar 22 E, Kalpana Area Ahimsa Bldg, SCO 82-83, Sector 17C 5-8-56/57 L. N. Gupta Marg D-277 Todarmal Marg, Banipark 117/418 B Sarvodaya Nagar Patliputra Industrial Estate Hantex Bldg (2nd Floor), Rly Station Road	AHMADABAD 380001 BANGALORE 560002 BHOPAL 462003 BHUBANESHWAR 751014 CHANDIGARH 160017 HYDERABAD 500001 JAIPUR 302006 KANPUR 208005 PATNA 800013 TRIVANDRUM 695001	2 03 91 2 76 49 6 27 16 5 36 27 2 83 20 22 10 83 6 98 32 8 12 72 6 28 08 32 27
	Deleted at Duth I Dute a	1. 10 A

Printed at Delhi Printers, Delhi, India